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## The Study of EADAS Elliptical Steel Damper Function in Seismic Resisting of Steel Frames

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### ABSTRACT

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The present article seek to investigate the function of elliptical steel damper function in steel frames seismic resisting. The elliptical damper is a kind of earthquake energy dissipater in structure which acts based on the steel yielding feature. Thus, this damper in frame lateral displacement mechanism is expected to have a satisfying hysteretic behavior in terms of energy absorption is on acceptable level. To study the EADAS (elliptical added damping and stiffness) elliptical damper function Abaqus software was used in seismic resisting of steel frames. To do the analysis one span- one floor which was modeled with eight kinds of suggested damper was used. The basis of the analysis method is based on the finite element method and a cyclic loading curve was used and the frame will be pushed to the failure of the dampers. The results of the present study indicates that through the addition of this damper with the damage focus in damper the structure elements remain in elastic zone and without any damage bear the lateral load. The damper causes an increase in ductility and the frame energy absorption hysteretic and it can be used for seismic resisting of steel frames.

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## 1. Introduction

Earthquake is one of the most well-known natural disasters which causes that many researchers do some research and

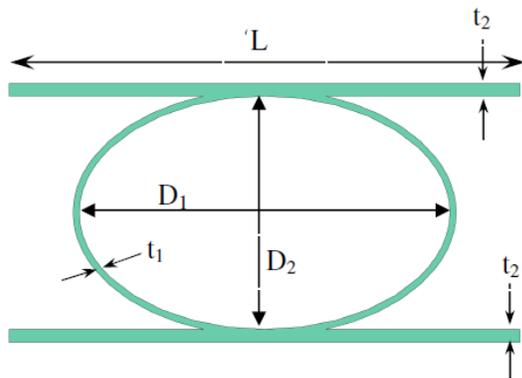
investigation on it each and every year in order to access Considerable progresses in this field but human knowledge in earthquake prediction either in terms of time and intensity is not still able to comment on



software studies have been used. Firstly, figure (1) frame was analyzed and designed by SAP2000 software [8], and the results of the design are presented in table (1). To study the damper application in the above frame, 8 kinds of damper which its features in figure (2) and table (2) are presented has been used. The frame has been modeled by this damper through Abaqus software [9], and has been analyzed through finite element.

**Table 1.** The features of the frame under study

Section type	Dimension
Beam	IPE 400
Columns (mm)	BOX 300*300*20
Braces (mm)	BOX 140*140*8
Floor height (m)	3.2
Column Span (m)	4.5



**Fig. 2.** EADAS Damper

**Table 2.** EADAS Dampers' Dimensions

Damper type	The Geometrical Properties				
	D <sub>1</sub> (cm)	D <sub>2</sub> (cm)	t <sub>1</sub> (cm)	t <sub>2</sub> (cm)	L (cm)
E <sub>1</sub>	20	10	0.5	1	40
E <sub>2</sub>	20	10	1	1	40
E <sub>3</sub>	20	15	0.5	1	40
E <sub>4</sub>	20	15	1	1	40
E <sub>5</sub>	30	15	0.5	1	40
E <sub>6</sub>	30	15	1	1	40
E <sub>7</sub>	30	20	0.5	1	40
E <sub>8</sub>	30	20	1	1	40

### 3. How to model with Abaqus software

To analyze this frame the Abaqus V 6.8.1 finite element software has been used [9].

In order to model the frame in the above software in the above software act as it follows:

The beam with the length of 60cm in solid 3D has been meshed. For modeling the joined plates of the damper to braces solid 3D element has been used and the mesh of 2mm is considered for it. A brace with the length of 15cms with 3D solid element with 2mm mesh has been modeled. In modeling beam and column the 3D wire element with section features of beam have been used and also for modeling braces 3D wire element with section features of Truss which works only in tension and compress has been used. To mesh, four-binding elements have been used. To connect beam to solid element rigid Connection is used to have a more unifying beam behavior in reciprocal loading. A joint is considered in the connection of the end of columns to the end of the beams. In the connection point of solid braces and wire Truss braces, rigid Connection has been used to make changes in shapes, places and times in the connection point of solid element to Truss element the same and virtually the brace member will behave unifyingly. To connect brace to the column's bottom joint connection is used. The damper is located between solid beam and Chevron type braces and its connection is considered like rigid to be able to observe its tension. Finally, a joint is used to define the column's leg support. Since, in the connection point and also the column's leg moment become zero and just frame behave exactly the same as simple frame in reciprocal cycle and the damper with changes in its non-linear shapes causes energy dissipation in system. For this purpose, the transmission in 1, 2, 3 are

closed ( $U_1=U_2=U_3=0$ ) and rotation are free ( $UR_1=UR_2=UR_3 \neq 0$ ) but rotations are free to ban support from having moment. In figure (3) the modeled frame in Abaqus software and in figure (4) the mesh method on solid part is shown.

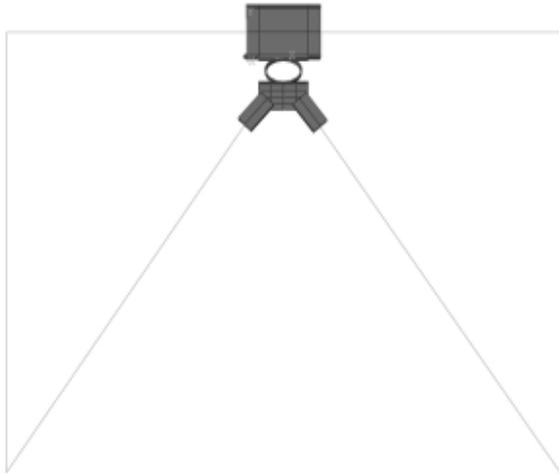


Fig. 3. Modeled frame in Abaqus software



Fig. 4. Mesh method on solid part

To investigate the frame behavior the cyclic load of figure (2) graph has been used and also the properties of the equipments which are used in the present research are defined as what is presented in table (3).

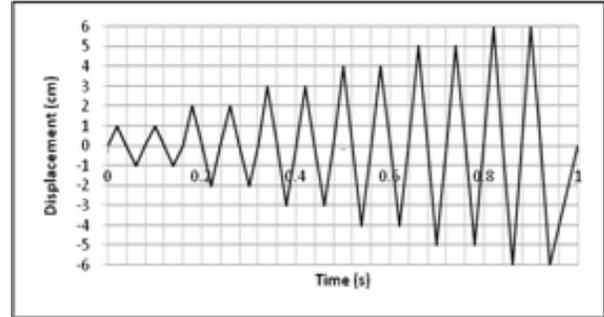


Fig. 5. The cyclic load curve

Table 3. Material properties

Steel type		ST-52	
Young module ( $\text{kg/cm}^2$ )		$2.1 \times 10^6$	
Poison ratio		0.3	
Stress-strain curve property	Plastic strain	0	0.02
	Yield stress	2400	4500
Mass ( $\text{kg/cm}^3$ )		$7.8 \times 10^{-6}$	

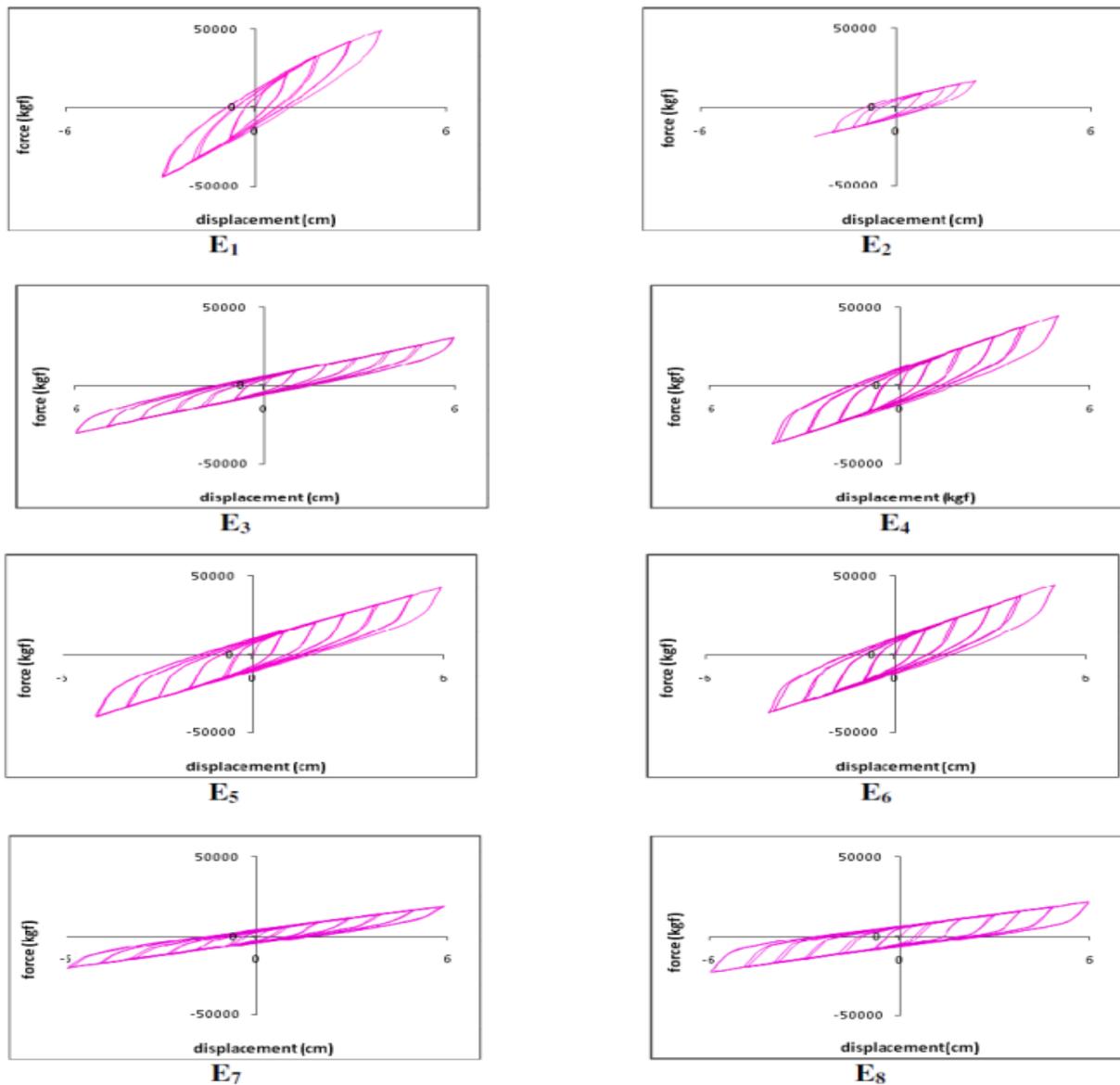
The hysteresis curves of the frames equipped with E1 to E8 type dampers are presented as it is explained in Figure (6).

#### 4. The analysis results

Generally, considering the graphs presented above and examining seismic parameters such as ductility coefficient, system equal damping, first stiffness, second stiffness and the amount of energy dissipation in ring, E4 type damper is suggested. With a more exact examination of this type of damper it is concluded that system damping increases from 2% to 26%. Even with the application of the weakest ring E7, damping increases up to 17% and also E4 type damper compare with other types include fatter hysteresis and this indicates more energy dissipation compare with other rings. Through applying this ring the frame hysteretic energy dissipation amount is equal to 52600 kg-cm. in terms of softness, it can be said that the

thicker the ring diameter, the structure will be softer, in the way that with an increase in rings' diameter, the hysteresis curve will be closer to the horizontal axis and the first stiffness (Elastic stiffness) and also the second stiffness decreases. Therefore, it can be understood that the ring's diameter is directly related to structure softness. Also,

by increasing the ring's diameter, the structure bearing capacity will be reduced and will have a reverse correlation. In the investigation of the non-linear of the damper E4 type, it can be said that the frame in 2mm displacement will show a non-linear behavior of the damper in the frame.



**Fig. 6.** The comparison of dampers' hysteresis graphs

## 5. Conclusion

In this paper, due to some studies and investigations on the impact of EADAS damper attachment to CHEVRON type brace steel frame we found that:

a) All works in elasto-plastic and plastic in damper have done and other members such as beam, columns and braces remain in the elastic region.

b) With the addition of EADAS damper to steel frame, the system damping will increase between the ranges of 17% to 26% which this percentage in E4 type reaches to its maximum that means 26%.

c) With a more exact study of the frames hysteresis curves we came to the conclusion that E4 curve is fatter than other curves, therefore, it shows a more energy dissipation in that frame.

In general it can be said that adding the damper ring to the desired structure can lead to the improvement of the seismic parameters of that frame in the time of earthquake. It is also possible to use this type of damper for resisting steel frames. Moreover, the damper is not a structural member. And with the focused damage on it, it can act like a structural fuse. After the earthquake, if the device (damper) damages, it can be replaced with lower cost and time.

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